

ex

Given

slab thickness = 120 mm

beam thickness = 700 mm

beam width = 250 mm

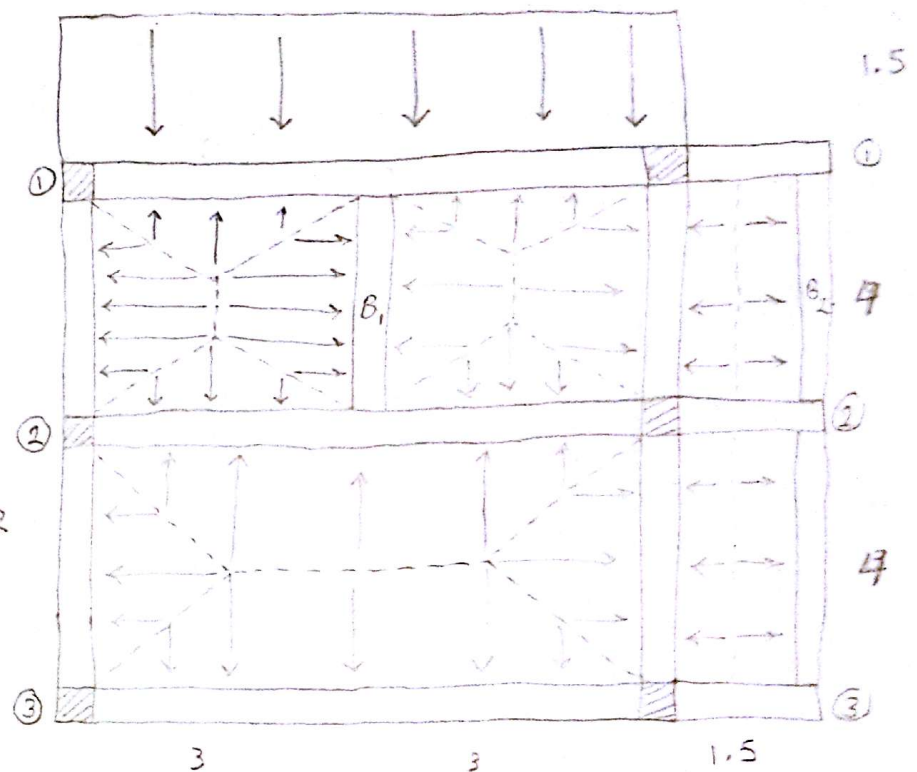
room f.c = 1.5 kN/m²

live load = 2.0 kN/m²

walls intensity = 3.0 kN/m²

Floor height = 3 m

section



Solution

* Draw load distribution of all slabs ?

* Calculate loads on beam AXE (2-2) ?

For Beam AXE (2-2)

$$\begin{aligned} \textcircled{1} \text{ o.w} &= \gamma_{\text{conc}} * b * (t - t_s) \\ &= 25 * 0.25 (0.7 - 0.12) = 3.6 \text{ kN/m} \end{aligned}$$

$$\begin{aligned} \textcircled{2} \text{ w.w} &= \gamma_{\text{brick}} * h_w \\ &= 3 * (3 - 0.7) = 6.9 \text{ kN/m} \end{aligned}$$

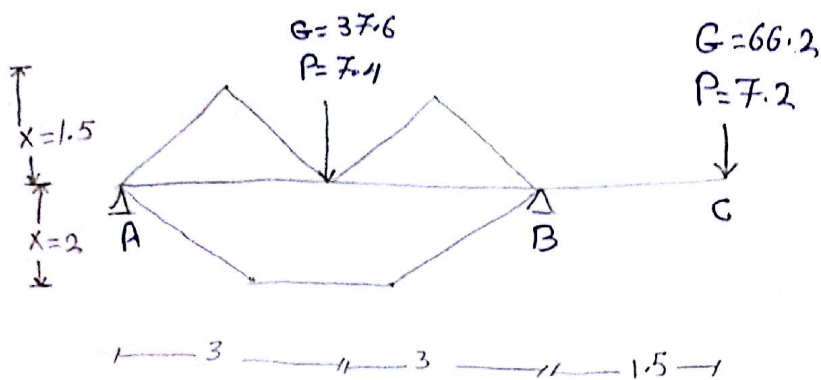
③ Loads on slab

$$\begin{aligned} \gamma_s &= \gamma_{\text{conc}} * t_s + \text{f.c} \\ &= 25 * 0.12 + 1.5 \\ &= 4.5 \text{ kN/m}^2 \end{aligned}$$

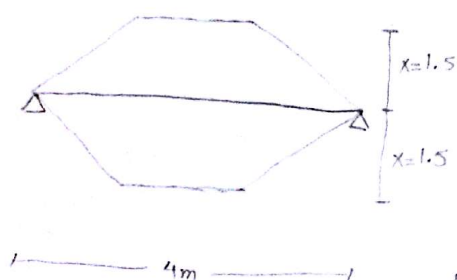
$$P_s = \text{L.L} = 2 \text{ kN/m}^2$$

2

Beam AX (2-2)



Beam (B)



$$\frac{L}{2X} = \frac{4}{2 \times 1.5} = 1.33$$

من الجدول 7.8

$$\alpha = 0.803$$

$$\beta = 0.615$$

عند حساب رد فعل كمرية يتم حساب (Load for shear) (moment) وقوة لها إذا كان لها (shear) (moment)

Load for shear

$$g = 0.7 + w_w + (g_s + \beta \cdot x) \times 2$$

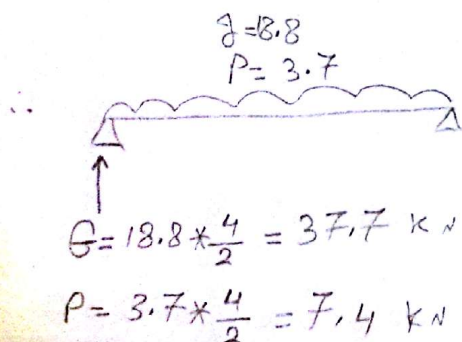
$$= 3.6 + 6.9 + (4.5 \times 0.615 \times 1.5) \times 2$$

$$= 18.8 \text{ kN/m}$$

$$p = (p_s + \beta \cdot x) \times 2$$

$$= 2 \times 0.615 \times 1.5 \times 2$$

$$= 3.7 \text{ kN/m}$$



$$G = 18.8 \times \frac{4}{2} = 37.7 \text{ kN}$$

$$P = 3.7 \times \frac{4}{2} = 7.4 \text{ kN}$$

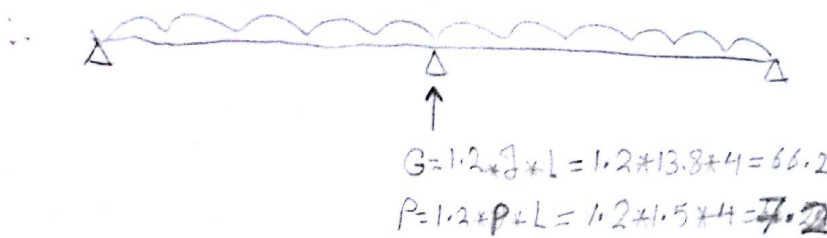
Beam 2



$$q = ow + ww + q_s \cdot x = 3.6 + 6.9 + 4.5 \times 0.75 = 13.8$$

$$P = P_s \cdot x = 2 \times 0.75 = 1.5$$

$$q = 13.8 \quad P = 1.5$$



Beam Axe (2-2)

Part BC

$$q = ow + wL = 3.6 + 6.9 = 10.5 \text{ kN/m}$$

$$P = 0.0$$

Part AB

$$\frac{\text{Area}}{\text{Span}} = \frac{2 \Delta}{\text{span}} = \frac{2 \times 0.5 \times 3 \times 1.5}{6} = 0.75$$

$$\text{for } \square \Rightarrow \frac{L}{2x} = 1.5 \quad \left\{ \begin{array}{l} \alpha = 0.853 \\ \beta = 0.667 \end{array} \right.$$

Load for Shear

$$q = ow + ww + q_s \left[\beta x + \frac{\text{Area}}{\text{Span}} \right]$$
$$= 3.6 + 6.9 + 4.5 \left[0.667 \times 2 + 0.75 \right] = 19.9$$

$$P = P_s \left[\beta x + \frac{\text{Area}}{\text{Span}} \right] = 2 \left[0.667 \times 2 + 0.75 \right] = 4.2$$

Load for Moment

$$q = ow + ww + q_s \left[\alpha x + \frac{\text{Area}}{\text{Span}} \right] = 21.6$$

$$P = 4.9$$

Load for shear

$$G=19.9$$

$$P=4.2$$

$$G=37.6$$

$$P=7.4$$

$$G=10.5$$

$$P=0.0$$

$$P=7.2$$

$$G=66.2$$

4

Load for moment

$$G=21.6$$

$$P=4.9$$

$$G=37.6$$

$$P=7.4$$

$$G=10.5$$

$$P=0.0$$

$$G=66.2$$

$$P=7.2$$

Case of total Load only

حالة ال ult ، ult فقط

* لما يكون موجود حمليت فقط على الكمره أمثريه في (1.4)

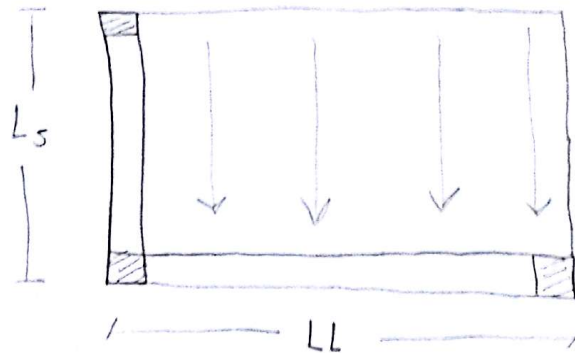
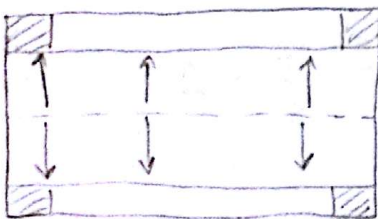
X

load dist. cases

1 one beam

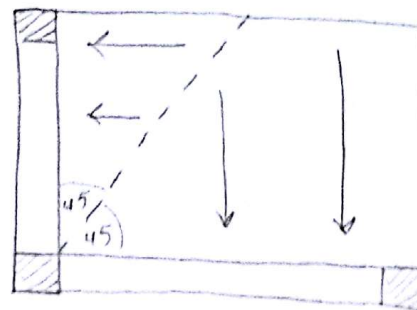
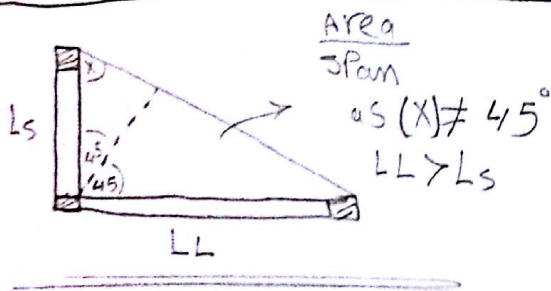


2 two beam

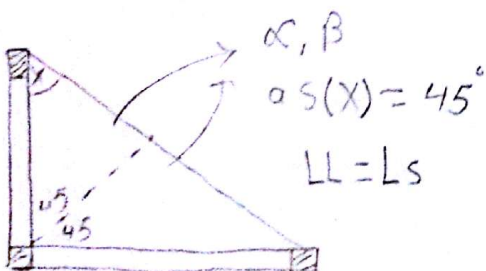


if $\frac{LL}{Ls} > 2.0$

الذي يذهب للكرة الخويلية



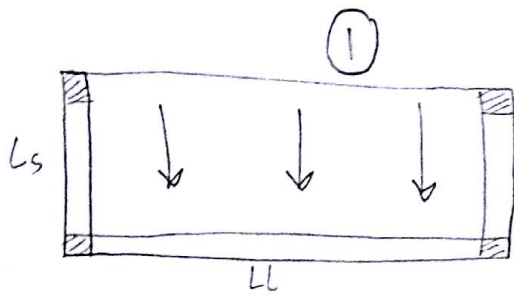
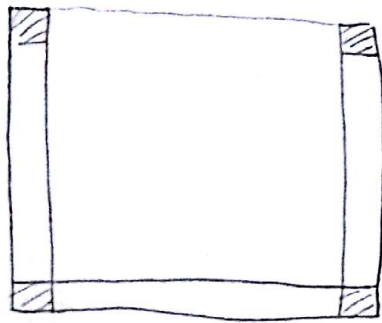
$\frac{LL}{Ls} < 2.0$



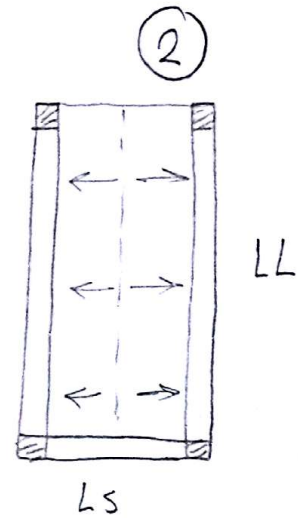
Area span لا يتوزع كالآتي ولا يتناسب

6

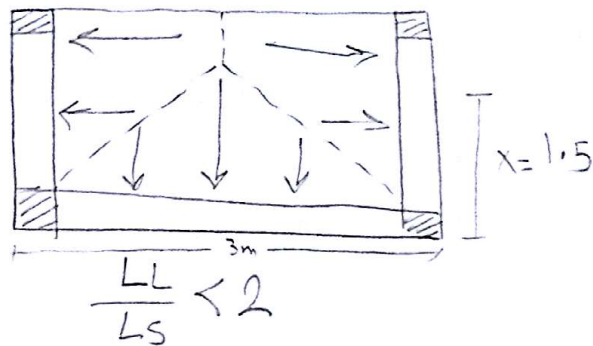
[3] three beams



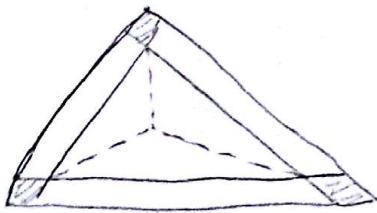
$$\frac{LL}{Ls} > 2$$



$$\text{if } \frac{LL}{Ls} > 2 \text{ (one way)}$$



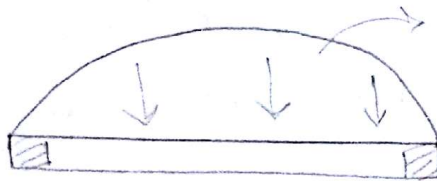
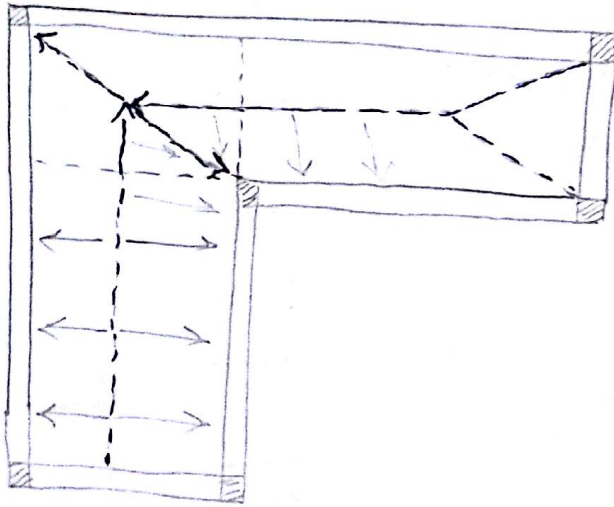
[3] الكمران المثلث



Area
Span

$$\text{Area} = \frac{1}{3} \Delta$$

7



$$\frac{\text{Area}}{\text{Span}}$$